

Out-of-Plane Behavior of Masonry Wall Strengthened Using Expanded Wire Mesh

S. Kanchidurai^{1*}, P. A. Krishanan², K. Baskar³, K. Saravana Raja Mohan⁴

^{1,4}School of Civil Engineering, SASTRA Deemed University, Thanjavur, Tamil Nadu, India.

^{2,3}Department of Civil Engineering, National Institute of Technology, Trichy, Tamilnadu, India.

*Corresponding author: kanchidurai@civil.sastra.edu

Abstract

Brick masonry is the principal part of the construction industry. This paper presents the result of the expanded wire mesh embedded into the masonry wall panel; expanded mesh (EM) is the inexpensive locally available material on the market. Through the experimental study, the primary affecting parameters recognized, and the methodology of the embedment details established, the preliminary survey was conducted as per the guidelines are given in the ASTM E518. Totally three numbers of control specimen and three numbers of EM embedded specimen are cast, and compressive strength test of masonry prism also evaluated to identify the crack propagation path. The test results turned out to be highly promising, out of plane damage was less in the EM embedded specimen also it was designated the cracks are reduced.

1. Introduction

In most of the buildings still used load-bearing masonry structures, during the earthquake out-of-plane loading severely affect the masonry part of the building. This paper presents the out-of-plane loading on masonry panel having 470x470x140 mm size with and without EM embedment. The EM is embedded on the surface of the brick for two courses first then it bends in the bed joint after again erected vertically on the brick surface as shown in figure 1. The previous research developed in the masonry structures are related to the embedment, which is wrapped in in-built in the plastering. shermi et al. have studied the study on the out-of-plane behavior of URM strengthened with welded wire mesh, the welded wire meshes are embedded by 2,2.5, and 3mm diameter steel mesh in this research ductility and out-of-plane flexural capacity significantly increased[1]. Kabir et al. investigated on GFRP retrofitted masonry panel for out-of-plane loading condition, four different type patterns of GFRP strips with different widths were used, this research, using the GFRP strips the masonry panels can significantly increase the ductility of force-displacement reaction, energy incorporation [2], shear behavior [3], resistance to total energy[4]. Mattia Colombo et al. have studied the masonry mortar rehabilitate by using fibre reinforced mortar by spray technology; results show the higher load carrying capacity and significant improvement in the deflection [5]. The application of Carbon fibre reinforced polymer (CFRP) strips applied on the critical surfaces of masonry building showing significant improvement in strength and performance [6]. Najif Ismail and Jason M. Ingham investigated on polymer textile reinforced mortar in masonry panel. In-plane and out-of-plane testing conducted on the strengthened specimen the results show up to 786% masonry strength was increased in in-plane testing, and there was a notable increment in out-of-plane testing [7]. Embedding twisted stainless steel bar in the existing masonry structure like retrofitting gives higher resistance by the bar during

out-of-plane testing, the twisted bar is inserted through drilling operations [8]. Masonry strengthening with CFRP in out-of-plane loading test significant improvement was noted, uniaxial and biaxial CFRP strips applied on the surface of the calcareous masonry panel and achieved significant flexural strength improvement[9], out-of-plane behavior of masonry unit checked by applying 50,100,150mm CFRP strips diagonally [10], in hollow block masonry 200mm CFRP strips applied closed longitudinal and transverse manner [11], near surface, mounted CFRP strips (NSM-CFRP) vertically mounted on masonry wall gives significant strength improvement [12]. Engineered Cementitious Composite (ECC) overlay on existing masonry structures are reliable in the strength improvement, 30mm overlay of ECC can give 336% of flexural strength improvement [13]. Masonry surface application of composites like GFRP wires, CFRP strips, and fibre reinforced mortar can considerably improve the flexural strength [14].



(a)



(b)

Fig. 1: EM embedded on the masonry panel for out-of-plane testing (a) elevation of masonry panel, (b) section of the masonry panel

2. Test Program and Technique

An innovative masonry panel of expanded wire mesh embedment was developed. To evaluate the masonry strength of embedded panels, the strength of chamber brick, the compressive strength of masonry prism and out-of-plane flexural test of masonry panels were carried out. To evaluate the material quality small-scale tests were conducted on cement, sand, bricks and expanded wires.

Materials

To make the Mesh embedded masonry panel cement mortar, bricks and expanded mesh were used to be locally available materials, and the galvanized expanded meshes are a cost-effective material having Rs.7 per square foot. According to ASTM C150 [15] type I Ordinary Portland Cement 53 grade was used as cement mortar provided by the local supplier. River sand used as Fine aggregate and the specific gravity of the sand was found 2.46 g/cc as per the procedure from ASTM C128[16]. Chamber burnt clay bricks are used to construct a masonry panel which is having average dimension 22.5 x 10.5 x 8.2 cm. The average compressive strength of clay brick was 12.01 MPa calculated as per guidelines are given by IS3495 [17], the water absorption of the brick 11.296 % comparing its dry weight. The Galvanized iron Expanded wire mesh having the diamond face of 0.71mm and the spacing of mesh 9.38 x 9.38 measured. The average yield strength of the EM wire is 445.75MPa.

Compressive Strength of Masonry Prism

The construction of masonry prism as shown in figure 2 with units lay in stack bond in stretcher position. Two number of brick and 10mm bed joint was used to make a masonry prism of 224 x 110 x 170 mm, EM was embedded in mortar bed joints of 1:3 (cement: sand) ratio. The entire compression test was carried out as per guidelines are given in the ASTM C1314 [18]. The compressive strength of each masonry prism strength calculated maximum compressive load applied divided by the net cross-sectional area of the prism, and compressive strength expressed in kilo Newton per millimeter square. Height to thickness ratio considered as the correction factor of the masonry prism, hence the final compressive strength of masonry prism multiplied by the correction factor as given by standard. Totally ten numbers of masonry prism constructed to compare the conventional masonry prism and EM embedded masonry prism. The masonry prism average height to thickness ratio (h_p / t_p) was 1.545 and the correction factor considered 0.862 from the ASTM C1314.



Fig. 2: Experimental set-up of compressive testing

Flexural Strength (out-of-Plane) of Masonry Panel

The masonry panel fabricated with six-bed joints and one or two perpend alternately, average dimension of masonry panel was 460 x 632 x 146 mm, totally six numbers of specimen fabricated in that three specimens are no mesh embedded units and three specimens are EM embedded (OPT-NE01-03 and OPT-NE01-03). The masonry mortar proportion 1:3 was maintained for six specimens, single layer of EM placed as shown in figure 1. So far the researchers were embedded only at the surface of the masonry, in this paper, the embedded mesh passed both surfaces of specimen and crossing bed joints. Three-point loading method and test method A was performed as per the guidelines are given in the ASTM E518 [16]. The entire test was carried out with 100 tone capacity Universal testing machine of controlled loading application; the speed of testing was maintained with uniform loading speed of 10kN per minute. All masonry panel curing was done for 28 days at the room temperature ($24 \pm 8^\circ\text{C}$). Figure 3 shows the experimental set-up of out-of-plane testing



Fig. 3: Experimental set-up of EM masonry panel out-of-plane loading

Construction Technique

Masonry solid wall panel constructed for 146 mm including 13.1mm plastering cover. The stretcher bond type single brick masonry panel was made with the influence of mesh embedment. The Expanded mesh was bonded for every two stretcher course in between the bed joints and erected vertically like zig-zag manner, Masonry mortar 1:3 (cement: sand) used in the masonry panel. In

practice this type of mesh bonding construction can be extended for a critical portion of the masonry wall, many previous research works was noticed only sticking GFRP mesh or another wrap on the masonry, but in this, the EM was taken differently.

3. Results and Discussion

Compressive Strength of Masonry Prism

Five specimens were tested for both mesh embedment and no mesh embedment (CSME 01-05 and CSNE 01-05). Application of compressive force gradually increased until the masonry prism sustains maximum, the maximum compressive force and the failure pattern noted to compare the incremental strength of EM embedded masonry. Table 1 shows the maximum sustained compressive load with and without EM embedment. Figure 4 shows the results of compressive strength of masonry prism. The

EM embedded masonry prism resist lager force than the conventional prism without embedment; almost 65% of average strength increment was noticed from the experimental results. The crack propagation was considerably varying with the applied load because of EM embedment in the prism, the applied compressive force scattered and the crack propagated towards the edges of the masonry prism whereas the conventional masonry prism shows the face shell separation.

Flexural Strength (out-of-Plane) of Masonry Prism

Four point flexural tests (out-of-plane) carried out according to ASTM E518-15; to evaluate the out-of-plane behavior of the masonry panel six tests were conducted. The maximum resistance offered by the specimen and the development of cracks was noticed, table 2 shows the results of flexural bending (out-of-plane) test.

Table 1: Compressive strength of masonry prism

specimen	hp/tp ratio	Correction factor	The compressive strength of prism f_{m} 'MPa.'	
			CS-NE	CS-ME
01	1.545	0.862	14.65	21.11
02	1.545	0.862	16.77	19.96
03	1.545	0.862	10.08	23.35
04	1.545	0.862	12.34	22.01
05	1.545	0.862	11.92	22.60
Average			13.152	21.806

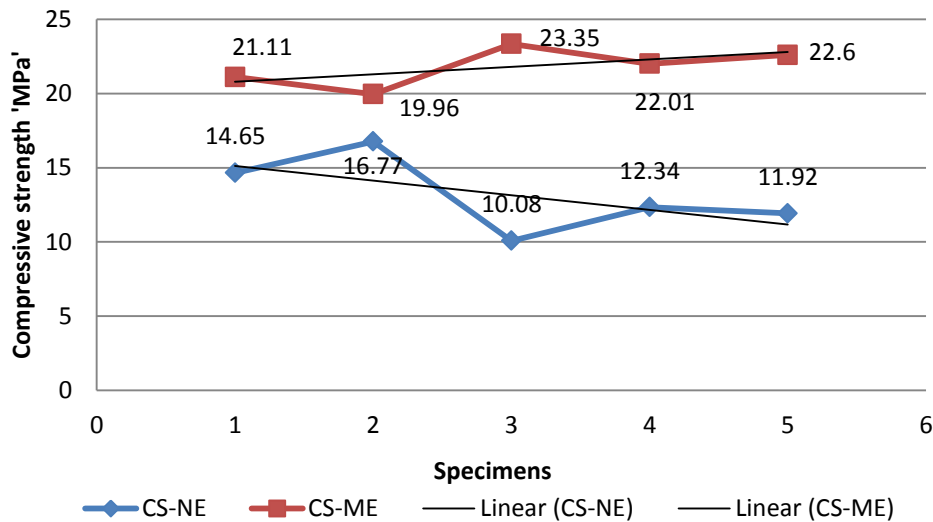


Fig. 4: comparison of embedded mesh and no embedment masonry prism

Table 2: Flexural strength (Out-of plane) of masonry panel

Specimen	Type	Maximum Load 'kN.'	Average	% of increase	Weight Ps 'kN.'	span	'R' 'Pa'
OPT-NE 01	Control	8.10	7.667	-	0.54	0.513m	0.617
OPT-NE 02	Control	7.50					0.574
OPT-NE 03	Control	7.40					0.566
OPT-ME 01	Embedded	12.30	11.37	48.24	0.613	0.931	
OPT-ME 02	Embedded	9.65					0.734
OPT-ME 03	Embedded	12.15					0.920

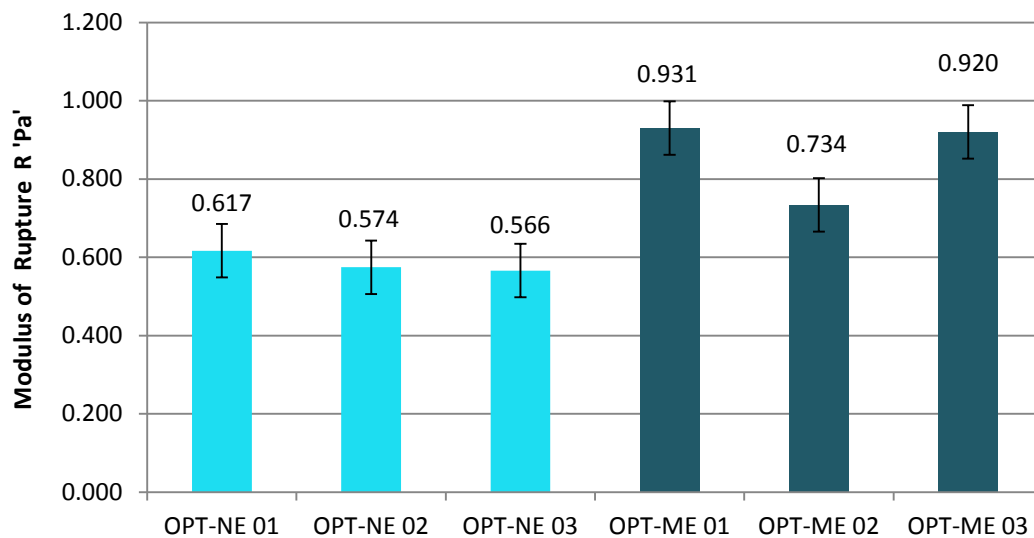


Fig. 5: Flexural strength of embedded and nonembedded masonry panels

4. Conclusion

The performance of the EM embedment is preventing the crack opening and after collapsing there is no complete separation noticed in the masonry panel.

1. The compressive strength of masonry prism was increased compared to no expanded mesh embedded panel (control masonry panel).
2. Failure cracks are propagated towards edge of the masonry prism at the time of compressive test and because of this specimen identified the splitting was reduced in the masonry
3. The experimental results reported here that the Modulus of rupture of masonry prism produced higher with embedment of expanded mesh

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